

# Blast Analysis on In-Filled Steel Tube Columns

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## ABSTRACT

In recent years, a large number of studies have been carried out to investigate the behaviours of concrete filled double skin steel tube (CFDST) members due to its increasing popularity in the construction industry. This project aims to study on ultra-high performance concrete filled double-skin tubes subjected to blast loading with cross section being square for both inner and outer steel tubes using ANSYS software. It is evident that the proposed CFDST column was able to withstand a large blast load without failure so that it has the potential to be used in high-value buildings as well as critical infrastructures. The steel tubes and concrete work together well and integrity of steel concrete interface is maintained. Steel tubes in inner and outer can acts as permanent formwork and primary reinforcement. ANSYS results shows that the CFDA column can withstand applied blast load.

**KEYWORDS:** CFDST Column, steel casing, Blast load

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## I. INTRODUCTION

Due to increase in the threat of terrorist activity, many efforts have been made to mitigate blast effect on structures so that they can remain unaffected by severe explosion accidents without catastrophic disaster, thus to decrease the number of human casualties. Using of blast barriers which are the indirect means to protect resistless infrastructures and people inside them are used widely. Although such indirect methods cannot productively prevent attacks initiated by suicide bombers or suitcase bombs as it was indicated by several recent terrorist attacks. Consequently, there is an immediate need to directly strengthen the blast-resistance of important structures through using new structural types or new

materials.

Recently, concrete-filled steel tubes, as a comparatively new steel concrete composite structure, have fascinated enormous amount of awareness in the civil engineering applied due to their high strength and excellent longevity. A concrete filled double-skin steel tube (CFDST) is usually constructed by filling concrete in-between two concentrically placed steel tubes. Passive confining pressure on the concrete filler resulted from the steel tubes which the main advantage of this structural type. The strength and plasticity of the concrete filler can be outstanding signify although the buckling of steel tubes can be retorted, if not completely prevented, by the concrete filler which can be caused due to confining pressure.

Usually, hollow steel tubes under transverse impact loading have failed a head of time due to severe local buckling or even transport before the material can develop its peak strength; if not, the concrete filler can effectively prevent steel buckling, consequently allowing the steel tube to progress its full strength and resulting in less deflection or structural destruction for the concrete filled steel tubes. Similarly as compared to conventional RC columns, breaching and spalling destruction of concrete are unlikely to occur in concrete filled steel tubes when subjected to blast loading. The majority of the recent studies on CFDST columns mainly use normal strength concrete filler. Nevertheless, modern trends of constructing high-rises and long-extent bridges have been made to blast load on structure at the distance of 1500mm from the structure.

**II. OBJECTIVES OF THE PROJECT**

- The present investigation aims to estimate the ultimate strength and study the behaviour of in-filled steel tube column.
- The in-filling of a steel tube column, concrete produces certain mechanical interaction, which is highly beneficial. The in-filled concrete increases its buckling strength under blast loading.
- To develop interaction diagram for various thickness of steel tube for different cross section of a column subjected to blast loading conditions.
- Responses of different structural systems are studied in terms of displacements due to beams and column.

**III. METHODS AND METHODOLOGY**

For the analysis, ANSYS 13 was used for the hollow beam having outer dimension 210mm x 210mm and inner dimension 100mm x 100mm and the length of the beam 2500mm was used.

Boundary condition is given for the column as simply supported. To ensure the both ends are simply supported for the column, all degree of freedom is selected. Outside the column, 50 Kg mass of explosive has been made to blast load on structure at the distance of 1500 mm from the structure.

Table.1 Material Properties

Property Name	Symbol	Values	Units
Grade	$f_{ck}$	25	N/mm <sup>2</sup>
Young's Modulus	E	29.58	GPa
Poisson ratio	$\nu$	0.19	---
Density	$\rho$	2500	kg/m <sup>3</sup>

**IV. VALIDATION OF MODEL**

Validation model consists of a beam having particular dimension. And we have material properties for concrete. In this project we have taken boundary condition of beam is simply supported beam with two point loading condition. The stress values which is obtained in ANSYS are compared with manual values and we have got errors of 5%.

Finally, by comparing manual values with ANSYS values, its found that, the elements, meshing and the boundary conditions that are used in the ANSYS models are accurate and can be used in determining the failure load of composite beam column

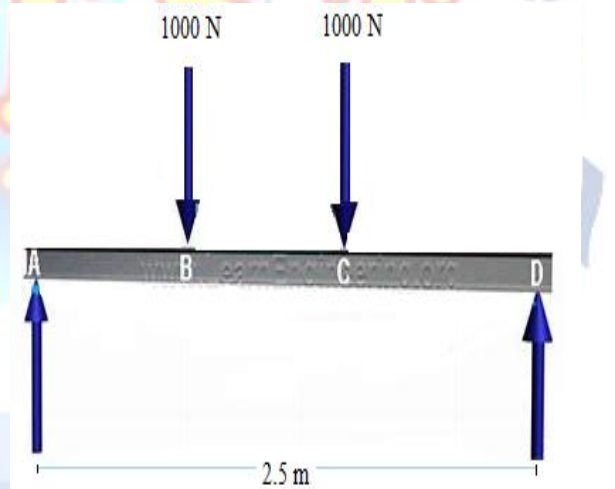


Fig.1 2D hollow beam section

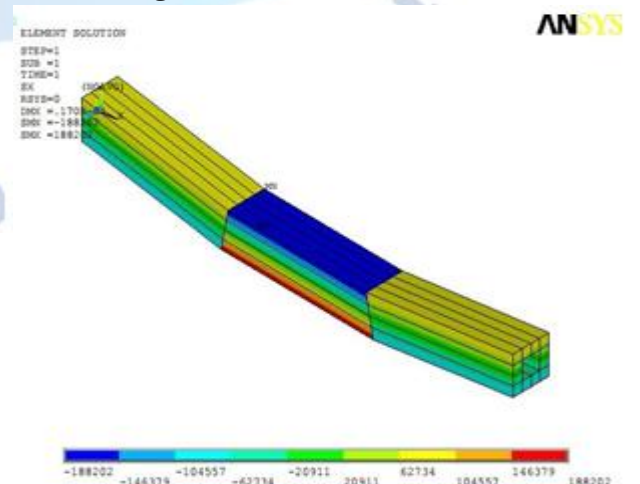


Fig.2 Stress values for hollow beam section



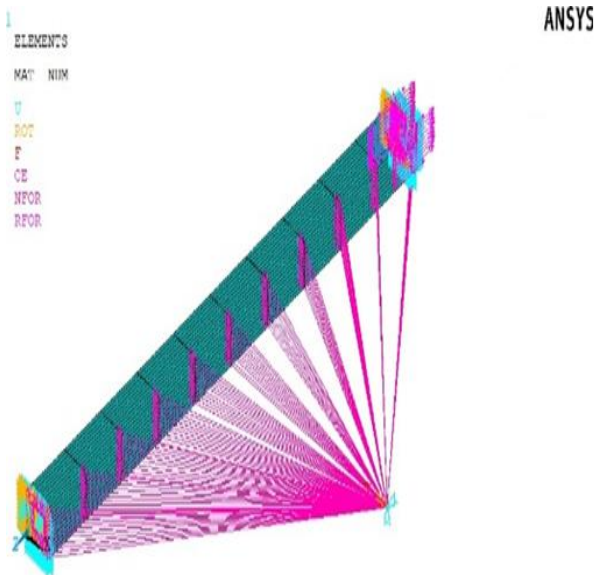


Fig 3 Boundry Conditions applied for beam

### V. RESULTS AND DISCUSSIONS

For the simulations carried out, we found shear stress in x direction, y direction, z direction, xy direction, yz direction and zx direction, among these direction zx direction shown the better results.

#### A. Inside casing

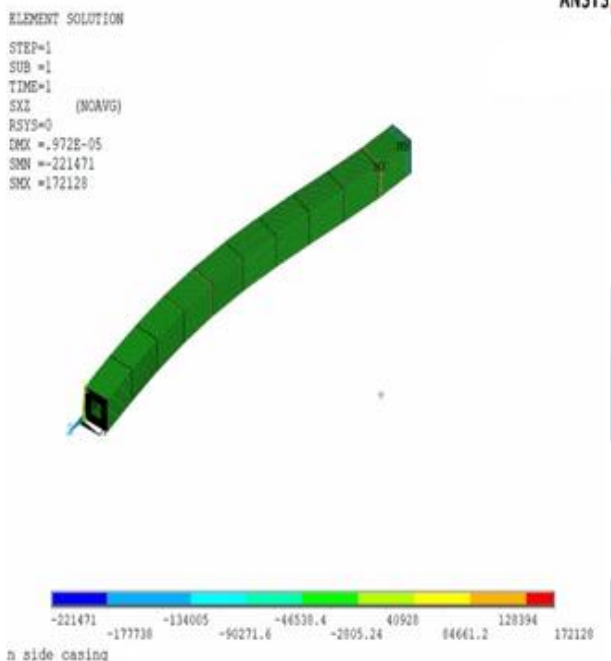


Fig.4 Inside casing along zx direction

Fig.4 shows the inside casing of thickness 5.5 mm along zx-direction. From ANSYS software, there is a displacement of 0.00972 mm in a column and shear stress is equal to 1,72,128 N/m<sup>2</sup> and the corresponding Force = 5869.56 N

#### B. No casing

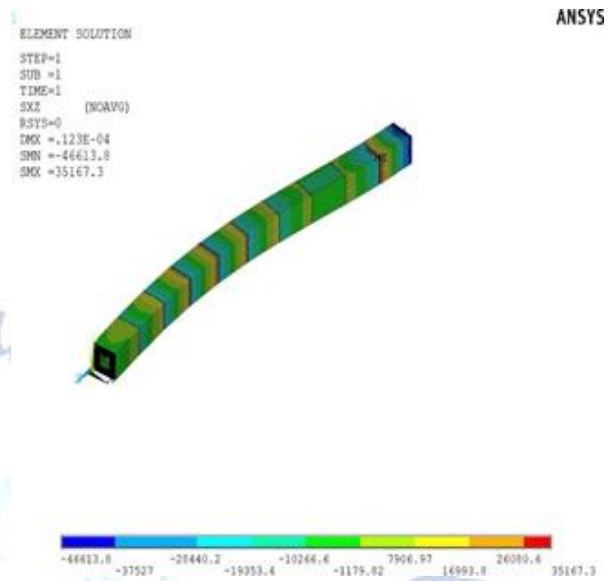


Fig.5 No casing along zx direction

Fig.5 shows the No casing along zx-direction. From ANSYS software, there is a displacement of 0.0123 mm in a column and shear stress is equal to 35,167.3 N/m<sup>2</sup> and the corresponding Force = 1199.20 N.

#### C. Out side casing

Fig.6 shows the outside casing of thickness 5.5 mm along zx-direction. From ANSYS software, there is a displacement of 0.0064 mm in a column and shear stress is equal to 1,13,529 N/m<sup>2</sup> and the corresponding Force = 3,871.34 N

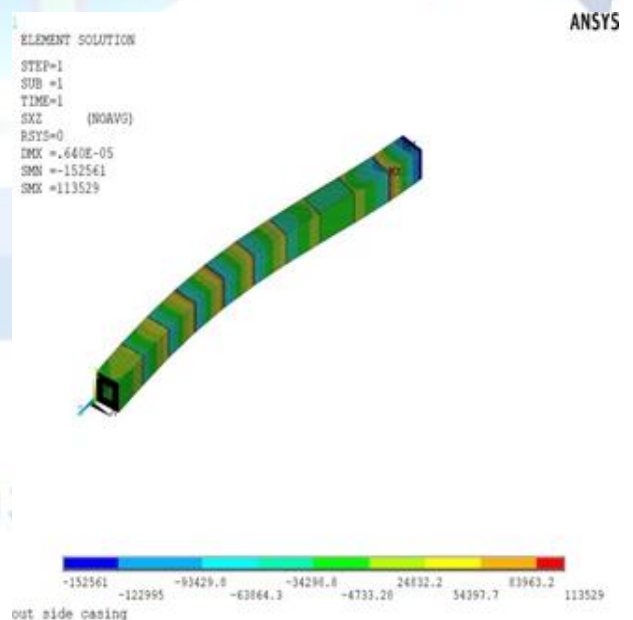


Fig.6 Out-side casing along zx direction

#### D. Two side casing

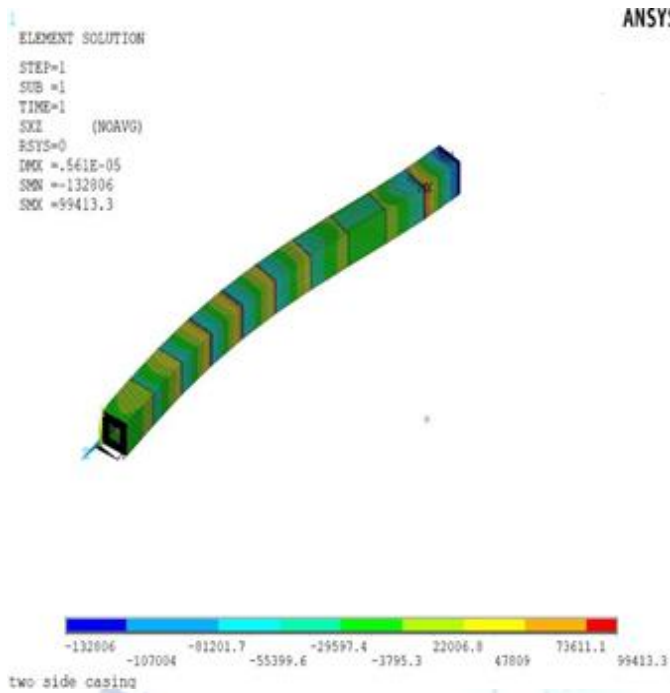


Fig.7 Two-side casing along zx direction

Fig.7 shows the two-side casing of thickness 5.5 mm along zx-direction. From ANSYS software, there is a displacement of 0.00561 mm in a column and shear stress is equal to 99,413.3 N/m<sup>2</sup> and the corresponding Force = 3,389.99 N.

#### VI. CONCLUSION

This paper has presented study on the performance of concrete-filled double-skin steel tube (CFDST) columns under blast loading using ANSYS software. The following conclusions can be drawn based on the software results:

- ANSYS results shows that the CFDA column can withstand applied blast load.
- By comparing with different casing of steel tubes, the use of 5.5mm thick steel tube for two-side casing in the CFDST specimens remarkably reduced the residual deflection where as the reduction in the maximum deflection was much less in percentage.
- While comparing to single sided tube casing, out side casing of steel tubes gives lesser displacement value compared to inner.
- Results of the analysis showed that shear stress is maximum along zx-direction and along this direction it is able to resist the blast load when the column was exposed to explosion.
- The Concrete filled double steel tube column resist more load compared to normal concrete column.

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