



# The Effect of Wood Ash on the Workability, Water Absorption, Compressive Strength in Cement Mortar

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## ABSTRACT

Many researches had been carried out to incorporate wood waste ash as a cement replacement material in the production of greener construction material (concrete/mortar) and also as a sustainable means of disposal for wood waste ash. Results of these researches indicated that wood waste ash can be effectively used as a cement replacement material for the production of structural concrete/mortar of acceptable strength and durability parameters.

In the present study water absorption, compressive test are performed in simulated environment of cement mortar, an overview of the work carried out with cement mortar in which cement is partially replaced by wood waste ash on several aspects such as the physical, chemical, strength and durability properties of mortar with wood waste ash and the result shows Compressive strength of 40mm cubes at 0% replacement of WWA is 24.07 N/mm<sup>2</sup> at 7days of curing and 28.13 N/mm<sup>2</sup> at 28 days of curing while at 10% replacement of WWA, the compressive strength is 17.28 N/mm<sup>2</sup> at 7 days of curing and 20.67 at 28 days of curing. It is clear from results that at replacement of 10%, compressive strength decreased by 26.52% in comparison to control mortar and also Water absorption capacity of mortar decreases with increase in percentage of wood ash. The reason behind this is that the particles of wood waste ash are finer than particles of cement which also acts like a filler material in hardened state.

**KEYWORDS:** Wood waste ash; Water Absorption Coefficient of Cement Mortar ;UTM Machine; Mortar with wood waste ash.

## 1. INTRODUCTION

This chapter contains the introduction, wood ash origin, wood ash applications, factors that affect quantity and quality of wood ash and objectives of wood ash in cement industry[1]. This chapter also deals with the problems that arise from combustion of wood, from cement industries and how we can handle wood ash economically as well as environment friendly. In the

current trends of energy production, power plants which run from biomass have low operational cost and have continuous supply of renewable fuel[2].

The thermal combustion/incineration reduces the mass and the volume of the waste up to a large extent, thus providing an environmentally safe and economically efficient way to manage the solid waste. Solid wood waste is commonly preferred as fuels over other

light/herbaceous and agricultural waste as their combustion produces comparatively less fly ash and other residual material[5].

### Origin of WoodAsh

The combustion/incineration of wood biomass produces wood ash as a by-product. A major problem arising from the usage of forest/agricultural waste product as fuel is related to the ash produced in significant amount after the combustion of such wastes[3]. It is commonly observed that the hardwood produce more ash than softwood and the bark and leaves generally produce more ash as compared to the inner part of the trees.

### Applications of Wood ash

Ash by-products obtained from combustion of wood biomass are generally used in land filling for embankments, road/highway construction[8]. It can be used as soil supplementing material to reduce the alkalinity of soil in agricultural lands. And it can also be used as raw material in the manufacturing of cement in cement industries[17].

As per current situation, approximately 70% of wood ash produced is managed by land filling, 20% of total wood ash produced is used as a soil supplement material for agricultural activity and the remaining 10% is implemented for other/miscellaneous applications as metal recovery and pollution control[10].

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**Udoeyo et al. (2006)** found the behavior of concrete containing wood ash at varying replacement percentages (0%, 5%, 10%, 15%, 20%, 25%, 30%). It was found out by experiments that concrete mix with WWA as a replacing material, decreases the workability of concrete[6,7]. This was due to that wood ash particles are smaller than that of cement particles due to which, wood ash particles has more specific surface area. Hence these particles absorb more water and ultimately, workability decreased.

Similarly, **Elinwa and Mehmood (2002)** found that when the cement is partially by wood waste ash obtained from open combustion/incineration of saw dust ash had the adverse effect on workability of grade 20 concrete. At constant w/c ratio of 0.565, when the percentage of replacement of cement is increased from

5% to 30% by weight of cement, slump value of concrete is decreased by 5-40mm with respect to control mix.

However, **Horsakulthai et al. (2011)** studied the effect of finely ground ash obtained from combustion of wood, rice husk ash, sugarcane bagasse waste (BRWA) as partial replacement of cement on chloride permeability of concrete. They used replacement percentages as 0%, 10%, 20%, 30% and 40% of total cement content by weight. It was concluded that incorporation of wood ash enhanced the chloride permeability. In the present study, The report presents an overview of the work carried out with cement mortar in which cement is partially replaced by wood waste ash on several aspects such as the physical, chemical, strength and durability properties of mortar with wood waste ash. This report shows the effect of wood ash on the workability, porosity, water absorption, compressive strength, flexural strength, thermal cycling, salt crystallization and rapid chloride permeability test etc.



**Fig 1: Wood Waste Ash Obtained from Open Burning.**

## 2. EXPERIMENTAL SETUP

### 1. Water absorption Test

To perform this test, we made prisms of size 40mm\*40mm\*160mm as per BS EN 1015:1999 Part-18, three for each replacement percentage (0, 5, 7.5, 10%). Take material in already given proportion as the cement and sand ratio is 1:3 and water/cement ratio is 0.50 and mixed it by hand or by epicyclic mixture. Prepare three specimens for each replacement percentage for testing at an age of 28 days.



**Fig 2: Moulds for Prisms of Size 40mm\*40mm\*160mm.**

**Testing:**

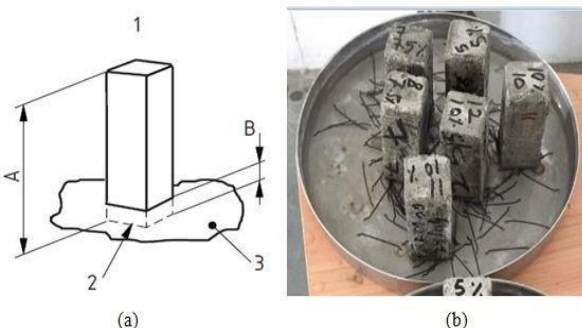
The prisms were dried at a temperature of  $60 \pm 5^\circ\text{C}$  till constant mass is attained. The constant mass is reached when the difference between two consecutive weighing is very very negligible. Drying was done at lower temperature instead of higher temperature so that organic matter in mortar, if present, should not be decomposed and so that structural water should not be destroyed.

After drying to constant mass, samples are coated with wax/epoxy on all four faces lengthwise as shown in fig 3.10 so that evaporation from sides does not take place.



**Fig 3: Prisms with Epoxy Coating**

Now, cut these samples into two halves (i.e., 80mm) and place the samples into tray as shown in fig 3.11 with cutting edge downwards in water. Put water upto 5 to 10mm depth into the tray to allow capillarity in samples.



**Fig 4: Immersion of Samples into Water Tray**

Remove the samples after 10 minutes to get readings. Weigh individually the specimen and note down as M1. Now, take the readings after 90 minutes and note.

**2. COMPRESSIVE STRENGTH:**

Compressive strength is defined as the resistance of mortar breaking under compressive load. It is the capacity of mortar to withstand under compressive load which break the specimen or fracture occurs in specimen. Here, compressive load is applied by Universal Testing Machine or by Compression Testing Machine.

To perform compressive strength test, we used two broken pieces (two halves) of flexural strength test (as mentioned in **BS EN 1015-11:1999**). When load was applied on prisms in flexure test, these prisms broke into two halves. These two halves were used for compression test. Two bearing plates of steel of size **40mm\*40mm\*10mm** are used to apply load on broken pieces.

**Testing:**

Test the samples at 7 days and 28 days after regular curing. **Universal Testing Machine (UTM)/ Compression Testing Machine (CTM)** of capacity 3000kN are used to test the compressive strength. Prisms, to be tested, were placed on UTM/CTM as shown in fig 5.

Bearing plates were applied, one above the prism and the other one below the prism, to uniformly distribute the load and to apply load on specified area (40mm\*40mm). Fig.5 shows the different arrangements of placing the sample between



**Fig 5: Different Arrangements of Bearing Plates on UTM**

The loading rate should be uniform (without shock) at a rate of **10N/sec to 50N/sec** so that failure can occur within a period of 30 sec to 90sec.

Calculate the compressive strength as the maximum/peak load carried by the specimen divided by its cross-sectional area (area of bearing plates). Record the strength of each specimen individually.

### 3. CALCULATIONS

UTM/CTM gives us peak load (in kN) value. We have to find out compressive strength/compressive stress (in N/mm<sup>2</sup>). To get compressive strength of broken halves (obtained from flexural strength test), we use following formula:

**Compressive Strength = Peak load/ area of bearing plates**

where,

Area of bearing plates = 40mm\*40mm = 1600mm<sup>2</sup>

To get compressive strength of cubes, we use following formula:

**Compressive Strength = Peak load/ area of cube [13,19]**

where, Area of cube = 50mm\*50mm = 2500mm<sup>2</sup>

### 4. RESULTS AND DISCUSSION

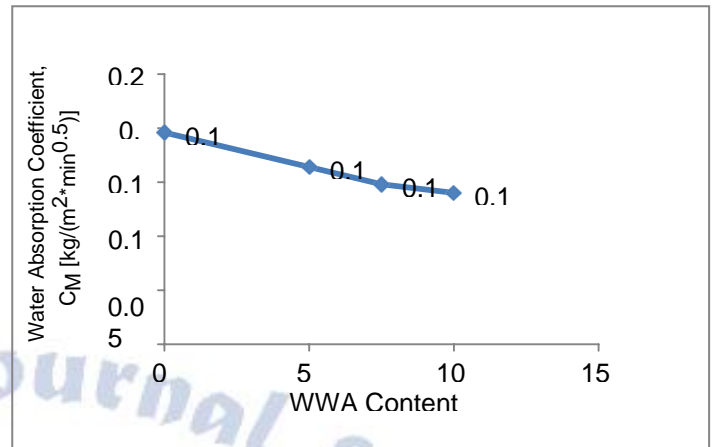
This chapter contains the outcomes of our experimental work. In this chapter, results of all the experiments are mentioned with their explanations and reasons behind the outcomes. This chapter gives the details of variation in results, graphical and tabulated representation of results.

#### 1. Water Absorption Capacity

Water absorption capacity is measured by calculating water absorption coefficient and for all replacement levels, water absorption coefficient is depicted in table 1 and Variation in water absorption coefficient [11,13], when replacement level of wood ash in cement is increased, is shown in fig 6.

**Table 1: Water Absorption Coefficient of Cement Mortar Containing WWA**

WWA Content (%)	Water Absorption Coefficient, C <sub>M</sub> [kg/(m <sup>2</sup> *min <sup>0.5</sup> )]	Mean of C <sub>M</sub>
0%	0.197	0.196
	0.184	
	0.209	
5%	0.172	0.164
	0.160	
	0.162	
7.5%	0.159	0.148
	0.145	
	0.140	
10%	0.147	0.140
	0.139	
	0.135	



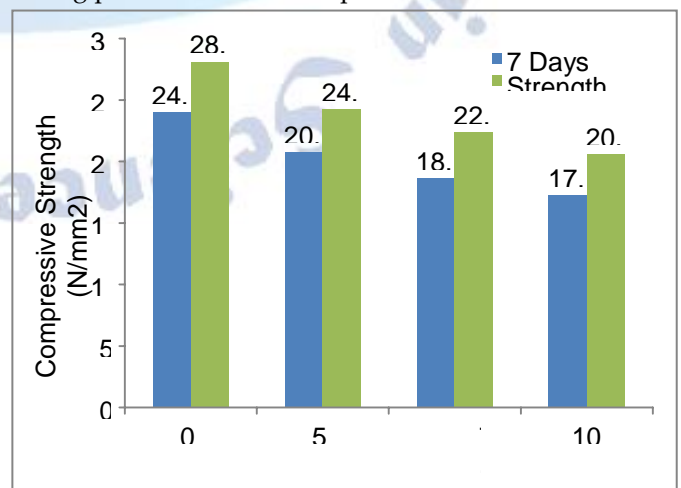
**Fig 6: Variation in Water Absorption Coefficient with Different WWA% Levels**

From figure 6, Water absorption capacity is inversely proportional to porosity and directly proportional to bulk density. Water absorption capacity of mortar decreases with increase in percentage of wood ash. The reason behind this is that the particles of wood waste ash are finer than particles of cement which also acts like a filler material in hardened state. As these particles are smaller than cement particles, they fill the void space between cement and sand particles [14,15]. As we increase the percentage of wood ash, we get compacted matrix which reduce water absorption capacity of hardened mortar.

#### 2. Compression Test

The Variation in compressive strength of broken pieces obtained from flexural test, for 7 days and 28 of curing, is shown in table 2. UTM gives load value, we have to convert these load values in stress values by dividing load value from area of samples [1,12,18].

From Figure 7, shows how the strength values differ with each other at 7 days curing period and at 28 days curing period at different replacement level



**Fig 7: Variation in Compressive Strength with Variation in WWA Content at 7 Days and at 28 Days (Broken Halves)**

the compressive strength of cubes of 50mm size with different replacement levels of wood ash in cement mortar illustrated in table 3.

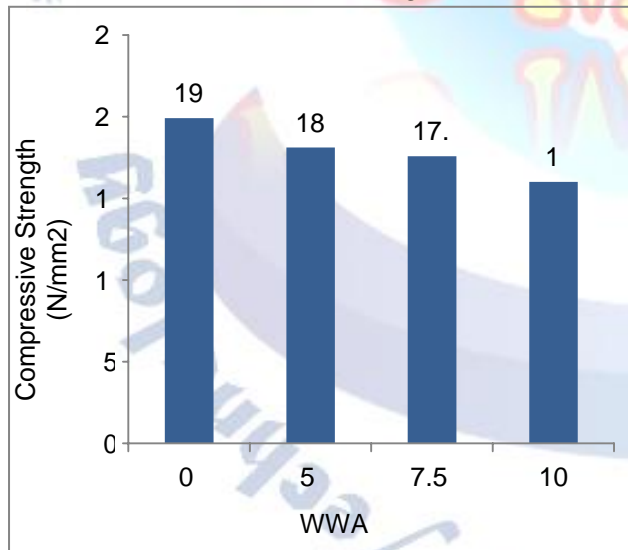
**Table 2: Compressive Strength of Mortar with WWA (Cubes)**

WWA Content (%)	28 days strength (N/mm <sup>2</sup> )
0%	19.9
5%	18.1
7.5%	17.56
10%	16

**Table 3: Compressive Strength of Mortar with WWA (Broken Halves)**

WWA Content (%)	7 days strength (N/mm <sup>2</sup> )	28 days strength (N/mm <sup>2</sup> )
0%	24.07	28.13
5%	20.81	24.23
7.5%	18.63	22.37
10%	17.28	20.67

The Variation in Compressive Strength of Cubes with Variation in WWA Content at 28 Days graph of decrease in compressive strength with increase in percentage of wood ash in mortar is shown in fig 8.



**Fig 8: Variation in Compressive Strength of Cubes with Variation in WWA Content at 28 Days**

Main components of cement are C<sub>3</sub>S, C<sub>2</sub>S, C<sub>3</sub>A and C<sub>4</sub>AF. During hydration process, C<sub>3</sub>S and C<sub>2</sub>S react with water to form calcium silicate hydrate (C-S-H) gel. C-S-H gel is important for good binding properties in mortar/concrete.

If silica content is more, more is the formation of C-S-H gel and more is the strength. But, from chemical composition of wood ash, silica is present in negligible amount which reduce the formation of C-S-H gel, hence reduction in compressivestrength[4,21,22].

## 5. CONCLUSIONS

On the basis of results and discussion following conclusions are drawn:

1. Compressive strength of 40mm cubes at 0% replacement of WWA is 24.07 N/mm<sup>2</sup> at 7days of curing and 28.13 N/mm<sup>2</sup> at 28 days of curing while at 10% replacement of WWA, the compressive strength is 17.28 N/mm<sup>2</sup> at 7 days of curing and 20.67 at 28 days of curing. It is clear from results that at replacement of 10%, compressive strength decreased by 26.52% in comparison to controlmortar.
2. Flexural strength results show that there is a significant decrease in flexural strength at upto 10% replacement level of wood ash. At 0% replacement of cement by WWA, the flexural strength is 7.1 N/mm<sup>2</sup> and at 10% replacement, strength is 5.82N/mm<sup>2</sup>.
3. If silica content is more, more is the formation of C-S-H gel and more is the strength. But, from chemical composition of wood ash, silica is present in negligible amount which reduce the formation of C-S-H gel, hence reduction in compressivestrength.
4. Water absorption capacity of mortar decreases with increase in percentage of wood ash. The reason behind this is that the particles of wood waste ash are finer than particles of cement which also acts like a filler material in hardened state.
5. Workability decreases with increase in replacement percentage of woodash.

## Conflict of interest statement

Authors declare that they do not have any conflict of interest.

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